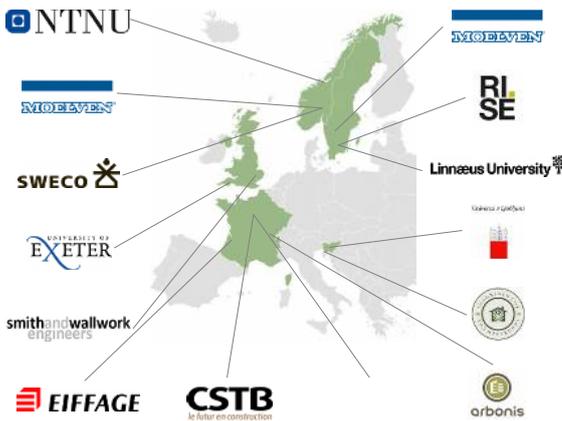
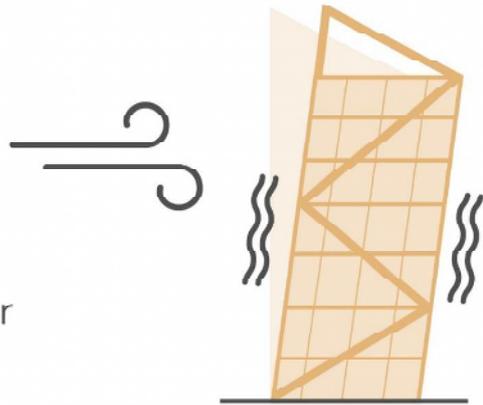


DynaTTB

Dynamic Response of Tall Timber Buildings under Service Load



DynaTTB is a project bringing together industrial and academic partners from 5 European countries (Slovenia, United Kingdom, Norway, Sweden and France).

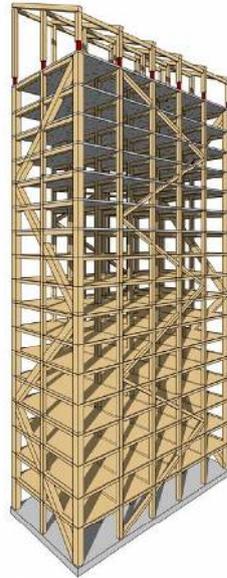


Project Dyna-TTB is supported under the umbrella of ERA-NET Cofund ForestValue by Vinnova – Sweden's Innovation Agency, Agence Nationale de la recherche, Ministry of Education, Science and Sport, The Research Council of Norway and Forestry Commission. ForestValue has received funding from the European Union's Horizon 2020 research and innovation programme under grant agreement N° 773324.

Dynamic Response of Tall Timber Buildings under Service Load (Dyna-TTB)

Starting point :

- The world's tallest wooden tower (Norway, Mjøstårnet, 85m) has been "weighted down" to meet comfort requirements.
- The top 7 floors have a 30cm thick concrete floor. The top floor exceeds the chosen comfort criteria. It is sold by specifying it to the buyer.
- Most all-wood buildings can meet this requirement (even for "small" heights of 4-5 floors).



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Tromsø, 10 stories, CLT elements



Yoker, 7 storeys, CLT



Eken, 6 storeys, Glulam



Mjøstårnet, 18 storeys, Glulam



Trinity College building, 4 stories, CLT elements



Treed-IT, 12 storeys,



Karantanika, 4 storeys, CLT



Hyperion, 18 storeys, hybrid + CLT



InnoRenew, 4 stories, CLT elements

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THE HYPÉRION TOWER – 56 M

- Located in Bordeaux
- 17 floors (R+16)
- Construction from March 2019 to June 2021
- Residential use
- Highest hybrid wood/concrete tower in France
- Large balconies



THE « TREED IT » TOWER – 36 M

- In Champs sur Marne
- 12 floors (R+11)
- Construction from mas 2018 to December 2020
- Student residence : Many interior partitions (rooms)



/ 5
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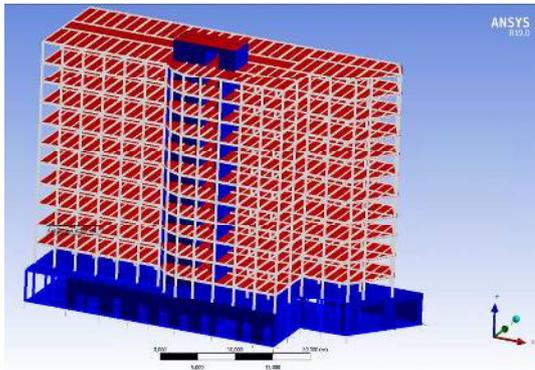


1st Treed-It campaign

Philosophy :

- The right material in the right place.
- Lighter structures.
- Optimization of material quantities (frugality).
- Favors the use of bio-sourced materials.
- Reduction of the carbon footprint of buildings.

What is special about these two towers is their hybrid character. The concrete cores, for reasons of fire protection in particular, are made of reinforced concrete. With this structural system, it is to be expected that the concrete core will mainly guide the lateral stiffness of the tower. Tests conducted at various times during construction have shown that this is not so simple.



Treed-It FEM

Concrete core all along the tower

Timber-concrete floors

CLT floors

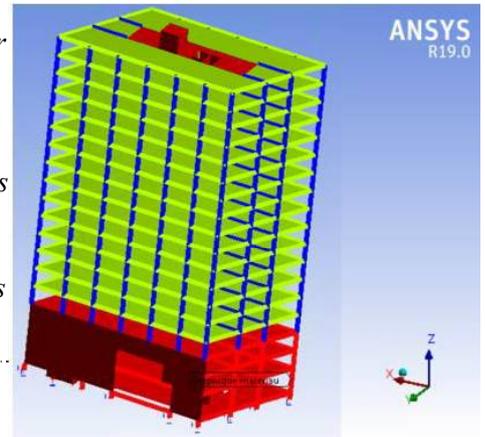
Timber columns

Steel columns

Timber beams

1st levels in concrete

Piles for foundations



Hyperion FEM

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With a heavy shaker input force is such that the amplitude of noise becomes negligible.

Two levels of amplitude, two levels of moving mass.

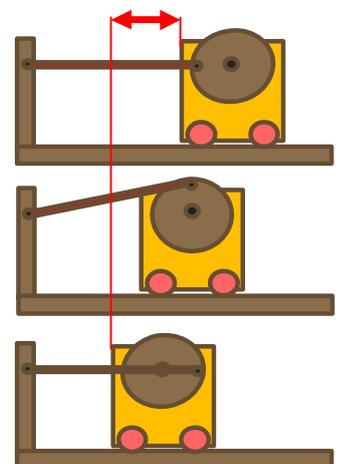
Lever arm can be set to 158mm or 248mm

Moving mass is 400kg; 150kg addition is possible

The applied force increases according to the frequency²

$$\Rightarrow F = M \cdot (2 \cdot \pi \cdot f)^2 \cdot A$$

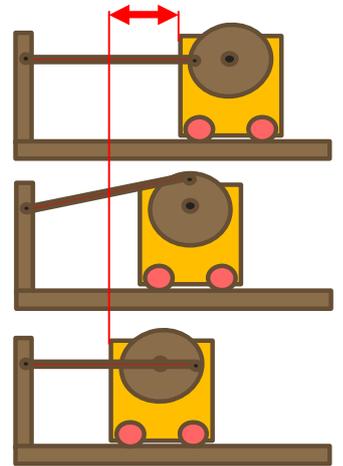
When the force of the shaker exceeds the frictional resistance, the exciter slides on the floor.



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Ballast per bag of sand



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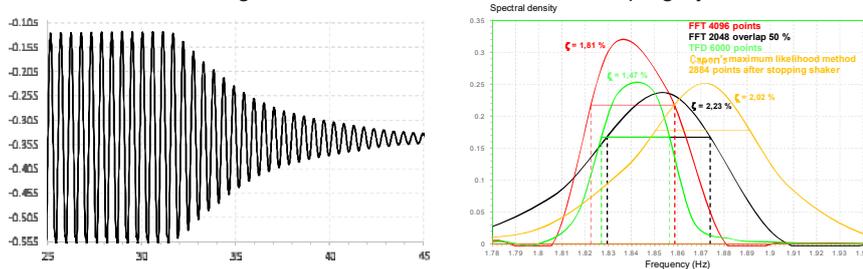
Location of sensors: 3 accelerometers on 3 different floors



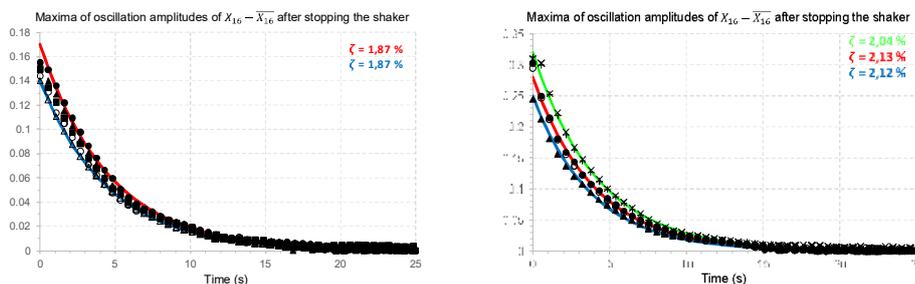
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- The measurement of vibrations under **ambient excitation**, by turbulent wind, nearby traffic or works inside the building, gives way to long lasting records (10 to 60 minutes) with the aim to identify modal frequencies. It provides a good insight of modal frequencies.
- An artificial excitation by a **mass shaker** delivering a **horizontal sinusoidal force**, with a **continuous and slow variation of the frequency** of which, called **swept sine test**. Because input force is controlled, the dynamic response of the building, i.e. accelerations measured at various locations, shows the frequencies of increased amplitude, pointing out the modal frequencies.
- An artificial excitation by a **mass shaker** delivering a **continuous horizontal sinusoidal force** at a **fixed frequency**. This **constant excitation test** must be brief to avoid beating, due to the unavoidable small difference between the frequency of the applied force and the modal frequency of the building which is excited. It was lasting 60s in this series of tests limiting data processing to time domain approach only.
- An **interrupted excitation** by the **mass shaker**, called a **shutdown test**, is used to **measure modal damping**. The initial excitation frequency is the one of a selected mode, then amplitude decreasing after the excitation has been stopped gives a good approach of modal damping.

Example of shutdown test record for bending mode and the evaluations of damping by various methods.



Examples of damping evaluation from repeated shutdown test records for bending mode with shaker located in the south corner of the building and with small amplitude (left) and on the east side with large amplitude (right)



increase of damping with amplitude on bending mode

“GOOD PRACTICE” MODELS HELP PREPARE FOR IN-SITU TRIALS

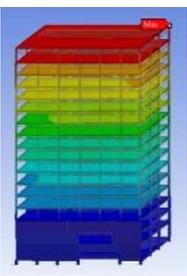
- > The model gives an estimate of the natural frequencies and modal deformations (choice and settings of the exciter, location of the sensors).
- > The model helps to convince the client before the tests: via a temporal simulation of the excitation by the machine and the demonstration that the amplitude of the deformation remains very limited (deformation verified by LVDT during tests).

IMPROVEMENT OF THE MODEL BY COMPARISON WITH TRIALS

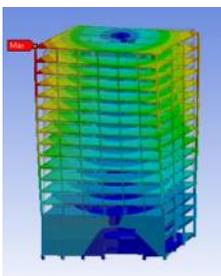
- > The measured frequencies and modal shapes are compared with the numerical results and an optimisation process is started.

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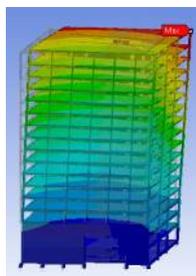
INITIAL FINITE ELEMENT MODEL



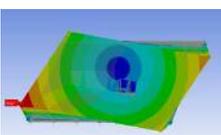
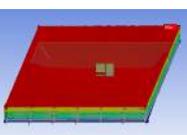
Bending 1



Torsion



Bending 2 + Torsion



The measured frequencies are directly compared with the numerical results

	f1	f2	f3
Experimental [Hz]	0.95	1.67	1.86
Initial [Hz]	0.76	1.52	1.37
Error [%]	-20.14	-9.21	-26.62

The modal shapes are compared with the numerical results (Modal Assurance Criterion)

		Experimental modes		
		0.95	1.67	1.86
FEM modes	0.76	0.940	0.083	0.009
	1.36	0.005	0.020	0.259
	1.52	0.026	0.818	0.658

An optimisation process can be started parameter by parameter. In addition to updating the material parameters used, or the connections between elements, it has sometimes been necessary to add components that were initially neglected.

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In simulation, lightweight partitions were initially considered more as a distributed mass than as a discrete element bringing a gain in rigidity to the structure.



1st round of testing



2nd round of testing

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On-site measurements were made **twice** for each of the two towers :

- 1/ With all the structural elements implemented (walls, floors, beams, etc.)
- 2/ After the addition of non-structural elements (light partitions, facade, balconies, etc.)

Treed-It	Mode 1 – bending X	Mode 2 – bending Y	Mode 3 - torsion
1 st round of testing (mass shaker)	1.37	1.54	1.69
2 nd round of testing (mass shaker)	1.39	1.49	1.62

Mass
+43%

Frequencies
-21% ???

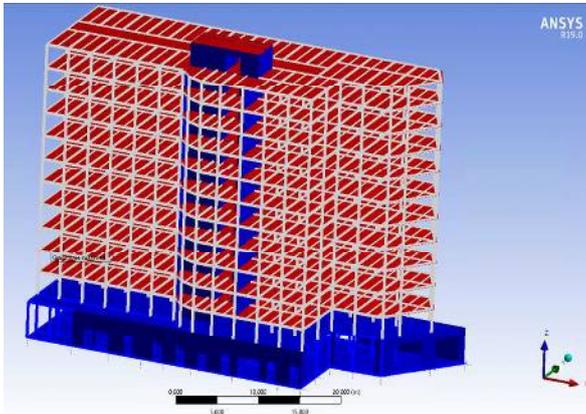
Hyperion	Mode 1 – bending X	Mode 2 – torsion	Mode 3 – bending Y
1 st round of testing (mass shaker)	0.95	1.67	1.88
2 nd round of testing (ambient)	0.95	1.50	1.81

Numerous partitions (student residence)

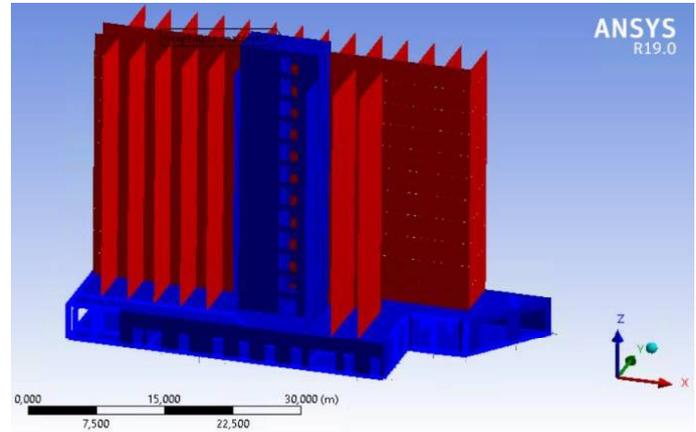
! Balcony weight (+700t)

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Ex ample on the Treed-It tower adding shell elements for the partitions



Initial model



View of the interior partitions

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Timber buildings (and hybrid building) are lightweight and subject to wind excitation, which poses a serious problem for occupant comfort ---► DynaTTB project.

In-situ tests of towers with a timber or hybrid structure were performed => providing modal frequencies and modal shapes that were compared with numerical simulations.

Damping was found to be amplitude dependent: a large amplitude excitation is needed to capture it, the FE model is of great help to prepare these tests.

The FE models need to be improved by :

- Optimising the parameters (initial values can be poorly estimated).
- Taking into account the stiffness of so-called "non-structural" elements as partitions or screeds, whose relative stiffness may have a role here in the overall stiffness of the structure

